

50 M³ ELEVATED STORAGE WATER TANK DESIGN REPORT

2nd September 2023

**Prepared by:
Sheilla Constance Apio**

ANALYSIS MODEL

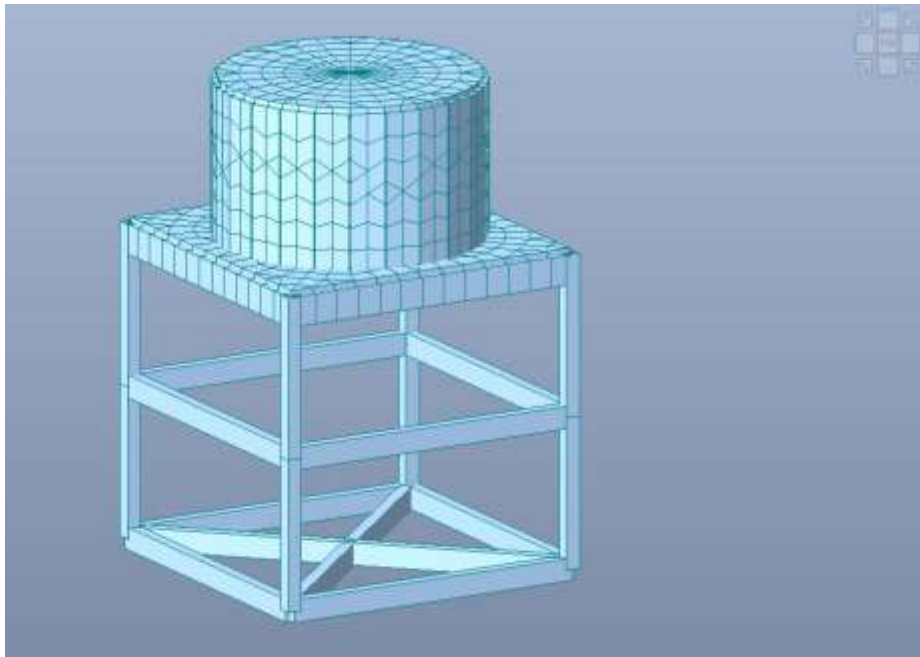


Figure 1 Analysis model

MODAL DISPLACEMENT

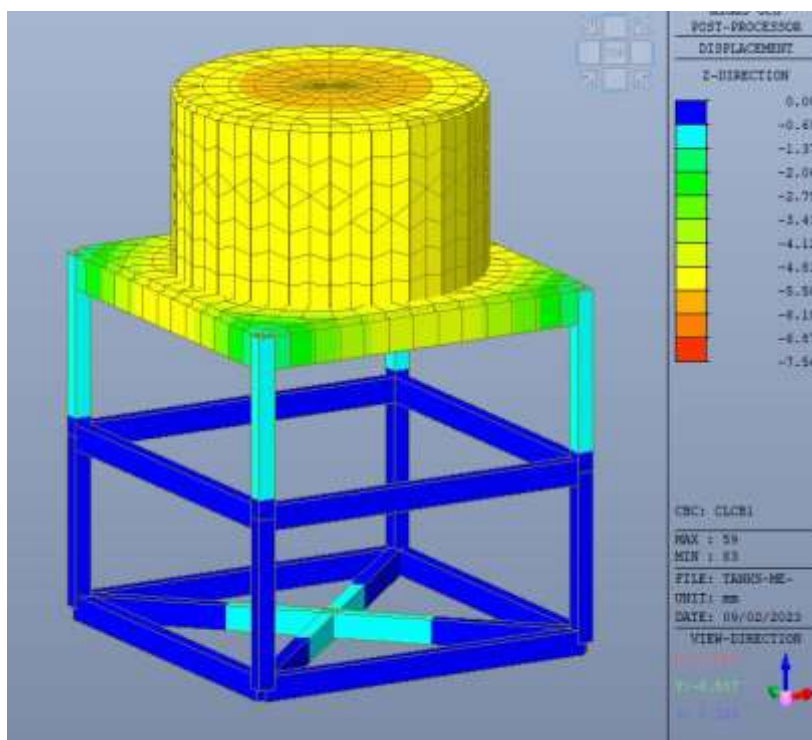


Figure 2 Modal displacement

LOAD CASES AND COMBINATIONS

No.	Name	Combination
1	cLCB1	1.35D + 1.5(1.0LL)
2	cLCB2	1.35D + 1.5(1.0LL) + 1.5(0.6)WL
3	cLCB3	1.35D + 1.5(0.7LL) + 1.5WL
4	cLCB4	1.35D + 1.5(1.0LL) - 1.5(0.6)WL
5	cLCB5	1.35D + 1.5(0.7LL) - 1.5WL
6	cLCB6	1.0D + 1.0(0.3L) + 1.0E
7	cLCB7	1.0D + 1.0(0.3L) - 1.0E
8	cLCB8	SERV :1.0D + (1.0LL)
9	cLCB9	SERV :1.0D + (1.0LL) + (0.6)WL
10	cLCB10	SERV :1.0D + (1.0LL) - (0.6)WL
11	cLCB12	SERV :1.0D + (0.7LL) - 1.0WL
12	cLCB13	SERV :1.0D + (0.5LL)
13	cLCB14	SERV :1.0D + (0.3LL) + (0.2)WL
14	cLCB15	SERV :1.0D + (0.3LL) - (0.2)WL
15	cLCB16	SERV :1.0D + (0.3LL)

D - DEAD LOAD

LL -LIVELOAD

WL - WIND LOAD (based on wind speed of 35 m/s)

E - EARTHQUAKE (based on a PGA of 0.064g)

SERV - Serviceability

LOADS

No.	Section	DEAD LOAD (kN/m2)	LIVE LOAD (kN/m2)
1.	Roof load	-1.2	-1.5

Pressure on the within the tank is based on water density of 10kN/m³

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	1158	BOT	0.0009	0.0010	55.4997(1)	64.1510	0.865	OK
	1071	TOP	0.0004	0.0010	25.3633(1)	64.1510	0.395	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 1158
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.030 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.4010 kN.
 M_Rd = Cc*(d-a/2) = 64.1510 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @200
 As_req = 0.0009 m^2/m. (0.0009 m^2/m.)
 M_Ed = 55.4997 kN-m./m.
 M_Rd = 64.1510 kN-m./m.
 RatM = M_Ed / M_Rd = 0.865 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.215

<< TOP >>

-. Information of Parameters.

Elem No. : 1071
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.030 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.4010 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 64.1510 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @200
 $A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (0.0004 \text{ m}^2/\text{m.})$
 $M_{Ed} = 25.3633 \text{ kN-m./m.}$
 $M_{Rd} = 64.1510 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.395 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.215$

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse		M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	1215	BOT	0.0009	0.0010		55.7113(1)	64.1510	0.868	OK
	1009	TOP	0.0004	0.0010		25.2422(1)	64.1510	0.393	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 1215
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.030 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.4010 kN.
 M_Rd = Cc*(d-a/2) = 64.1510 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @200
 As_req = 0.0009 m^2/m. (0.0009 m^2/m.)
 M_Ed = 55.7113 kN-m./m.
 M_Rd = 64.1510 kN-m./m.
 RatM = M_Ed / M_Rd = 0.868 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.215

<< TOP >>

-. Information of Parameters.

Elem No. : 1009
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.030 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.4010 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 64.1510 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @200
 $A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (0.0004 \text{ m}^2/\text{m.})$
 $M_{Ed} = 25.2422 \text{ kN-m./m.}$
 $M_{Rd} = 64.1510 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.393 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.215$

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	4081	BOT	0.0002	0.0004	6.94260(1)	26.5133	0.262	OK
	4213	TOP	0.0002	0.0004	8.29102(1)	26.5133	0.313	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4081
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1568 kN.
 M_Rd = Cc*(d-a/2) = 26.5133 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0002 m^2/m. (0.0002 m^2/m.)
 M_Ed = 6.9426 kN-m./m.
 M_Rd = 26.5133 kN-m./m.
 RatM = M_Ed / M_Rd = 0.262 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.084

<< TOP >>

-. Information of Parameters.

Elem No. : 4213
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1568 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 26.5133 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0002 \text{ m}^2/\text{m.} \quad (0.0002 \text{ m}^2/\text{m.})$
 $M_{Ed} = 8.2910 \text{ kN-m./m.}$
 $M_{Rd} = 26.5133 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.313 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.084$

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	4081	BOT	0.0002	0.0004	6.95893(1)	26.5133	0.262	OK
	4165	TOP	0.0002	0.0004	8.37972(1)	26.5133	0.316	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4081
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1568 kN.
 M_Rd = Cc*(d-a/2) = 26.5133 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0002 m^2/m. (0.0002 m^2/m.)
 M_Ed = 6.9589 kN-m./m.
 M_Rd = 26.5133 kN-m./m.
 RatM = M_Ed / M_Rd = 0.262 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.084

<< TOP >>

-. Information of Parameters.

Elem No. : 4165
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1568 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 26.5133 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0002 \text{ m}^2/\text{m.} \quad (0.0002 \text{ m}^2/\text{m.})$
 $M_{Ed} = 8.3797 \text{ kN-m./m.}$
 $M_{Rd} = 26.5133 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.316 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

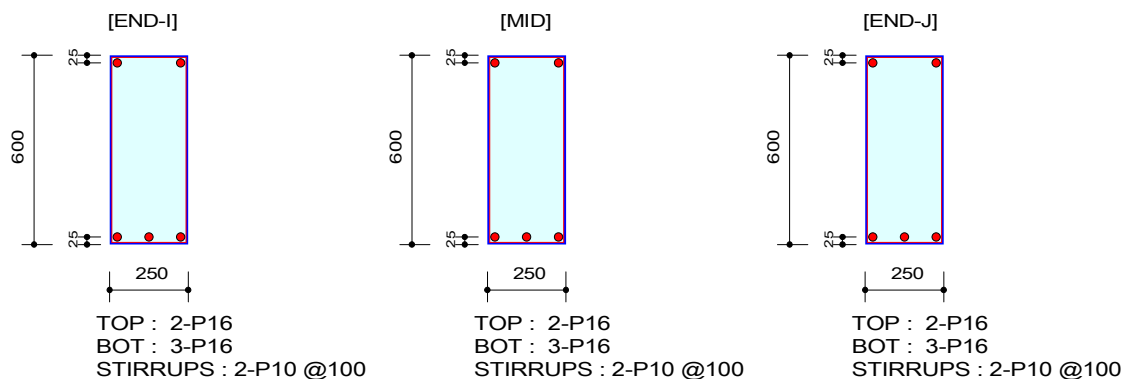
$x/d = 0.084$

	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : $f_{ck} = 0.02$, $f_{yk} = 0.46$, $f_{yw} = 0.25 \text{ kN/mm}^2$
 Section Property: 600x250 loading beams (No : 5) Unit System : kN, mm
 Beam Span : 8980.26 mm

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	29673.70	7453.14	29812.54
Factored Strength (M_Rd)	88980.14	88980.14	88980.14
Check Ratio (M_Ed/M_Rd)	0.3335	0.0838	0.3350
Neutral Axis (x/d)	0.0533	0.0533	0.0533
(+) Load Combination No.	4	4	4
Moment (M_Ed)	52231.77	115972.15	68352.51
Factored Strength (M_Rd)	132436.44	132436.44	132436.44
Check Ratio (M_Ed/M_Rd)	0.3944	0.8757	0.5161
Neutral Axis (x/d)	0.0781	0.0781	0.0781
Using Rebar Top (As_top)	402.1200	402.1200	402.1200
Using Rebar Bot (As_bot)	603.1800	603.1800	603.1800

4. Shear Capacity

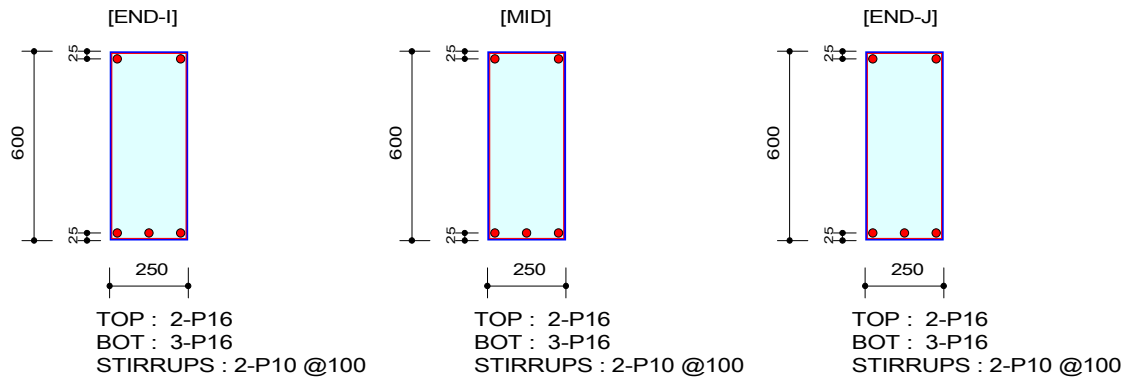
	END-I	MID	END-J
Load Combination No.	4	4	4
Factored Shear Force (V_Ed)	165.92	33.97	163.29
Shear Strength by Conc.(V_Rdc)	55.73	55.73	55.73
Shear Strength by Rebar.(V_Rds)	176.72	176.72	176.72
Shear Strength by Rebar.(V_Rdmax)	476.10	476.10	476.10
Using Shear Reinf. (Asw)	1570.8000	1570.8000	1570.8000
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	2.9772	0.6095	2.9301
Shear Ratio by (V_Rds ; V_Rdmax)	0.9389	0.1922	0.9240
Check Ratio	0.9389	0.6095	0.9240

	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code	: Eurocode2:04	Unit System	: kN, mm
Material Data	: fck = 0.02, fyk = 0.46, fyw = 0.25 kN/mm^2		
Section Property:	600x250 tie beams (No : 4)	Beam Span	: 6350 mm

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	19826.00	4957.06	19828.22
Factored Strength (M_Rd)	88980.14	88980.14	88980.14
Check Ratio (M_Ed/M_Rd)	0.2228	0.0557	0.2228
Neutral Axis (x/d)	0.0533	0.0533	0.0533
(+) Load Combination No.	4	4	4
Moment (M_Ed)	72094.46	105833.10	72266.40
Factored Strength (M_Rd)	132436.44	132436.44	132436.44
Check Ratio (M_Ed/M_Rd)	0.5444	0.7991	0.5457
Neutral Axis (x/d)	0.0781	0.0781	0.0781
Using Rebar Top (As_top)	402.1200	402.1200	402.1200
Using Rebar Bot (As_bot)	603.1800	603.1800	603.1800

4. Shear Capacity

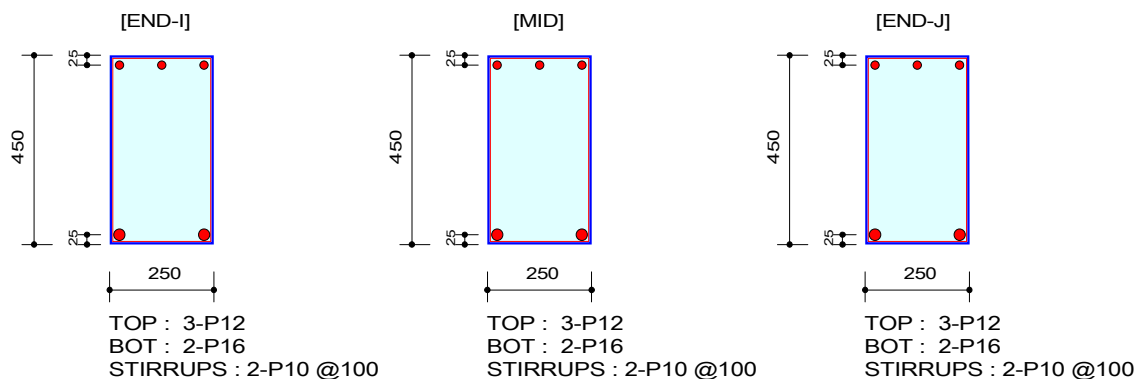
	END-I	MID	END-J
Load Combination No.	4	7	4
Factored Shear Force (V_Ed)	152.23	35.70	152.43
Shear Strength by Conc.(V_Rdc)	55.73	55.73	55.73
Shear Strength by Rebar.(V_Rds)	176.72	176.72	176.72
Shear Strength by Rebar.(V_Rdmax)	476.10	476.10	476.10
Using Shear Reinf. (Asw)	1570.8000	1570.8000	1570.8000
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	2.7316	0.6405	2.7352
Shear Ratio by (V_Rds ; V_Rdmax)	0.8614	0.2020	0.8626
Check Ratio	0.8614	0.6405	0.8626

	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code	: Eurocode2:04	Unit System	: kN, mm
Material Data	: fck = 0.02, fyk = 0.46, fyw = 0.25 kN/mm^2		
Section Property:	450x250 tie beams (No : 6)	Beam Span	: 6350 mm

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	14056.13	3515.42	14061.69
Factored Strength (M_Rd)	55164.63	55164.63	55164.63
Check Ratio (M_Ed/M_Rd)	0.2548	0.0637	0.2549
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	4	5	4
Moment (M_Ed)	7028.06	5894.74	7030.84
Factored Strength (M_Rd)	65137.57	65137.57	65137.57
Check Ratio (M_Ed/M_Rd)	0.1079	0.0905	0.1079
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As_top)	339.3000	339.3000	339.3000
Using Rebar Bot (As_bot)	402.1200	402.1200	402.1200

4. Shear Capacity

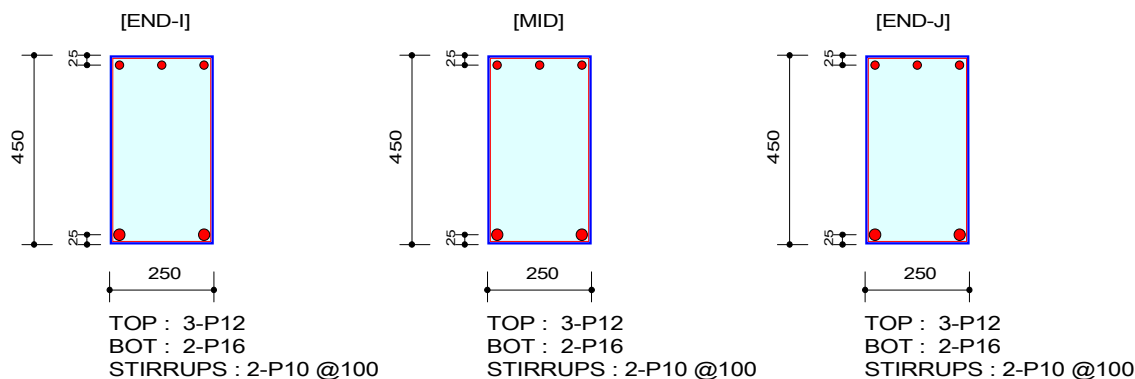
	END-I	MID	END-J
Load Combination No.	7	7	7
Factored Shear Force (V_Ed)	27.88	18.95	27.88
Shear Strength by Conc.(V_Rdc)	42.21	42.21	42.21
Shear Strength by Rebar.(V_Rds)	130.62	130.62	130.62
Shear Strength by Rebar.(V_Rdmax)	351.90	351.90	351.90
Using Shear Reinf. (A _{sw})	1570.8000	1570.8000	1570.8000
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.6604	0.4489	0.6604
Shear Ratio by (V_Rds ; V_Rdmax)	0.2134	0.1450	0.2134
Check Ratio	0.6604	0.4489	0.6604

	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : $f_{ck} = 0.02$, $f_{yk} = 0.46$, $f_{yw} = 0.25 \text{ kN/mm}^2$
 Section Property: 450x250 ground beams (No : 7) Unit System : kN, mm
 Beam Span : 8980.26 mm

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	5	7	5
Moment (M _{Ed})	25516.58	0.00	25516.58
Factored Strength (M _{Rd})	55164.63	55164.63	55164.63
Check Ratio (M _{Ed} /M _{Rd})	0.4626	0.0000	0.4626
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	5	5	5
Moment (M _{Ed})	3189.57	12758.29	3189.57
Factored Strength (M _{Rd})	65137.57	65137.57	65137.57
Check Ratio (M _{Ed} /M _{Rd})	0.0490	0.1959	0.0490
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As _{top})	339.3000	339.3000	339.3000
Using Rebar Bot (As _{bot})	402.1200	402.1200	402.1200

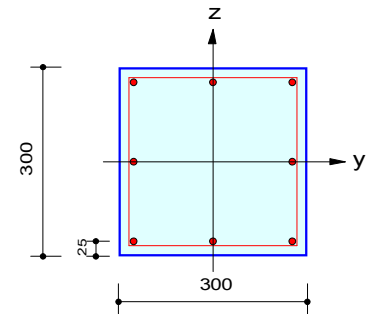
4. Shear Capacity

	END-I	MID	END-J
Load Combination No.	5	5	5
Factored Shear Force (V _{Ed})	17.05	8.52	17.05
Shear Strength by Conc.(V _{Rdc})	42.21	42.21	42.21
Shear Strength by Rebar.(V _{Rds})	130.62	130.62	130.62
Shear Strength by Rebar.(V _{Rdmax})	351.90	351.90	351.90
Using Shear Reinf. (Asw)	1570.8000	1570.8000	1570.8000
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.4039	0.2020	0.4039
Shear Ratio by (V _{Rds} ; V _{Rdmax})	0.1305	0.0653	0.1305
Check Ratio	0.4039	0.2020	0.4039

	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Condition

Design Code : Eurocode2:04 UNIT SYSTEM kN, mm
 Member Number: 92 (PM), 87 (Shear)
 Material Data : fck = 0.02, fyk = 0.46, fyw = 0.25 kN/mm²
 Column Height : 3250 mm
 Section Property: 300x300 columns (No : 2)
 Rebar Pattern : 8 - 3 - P16 Ast = 1608.48 mm² (Rho = 0.018)



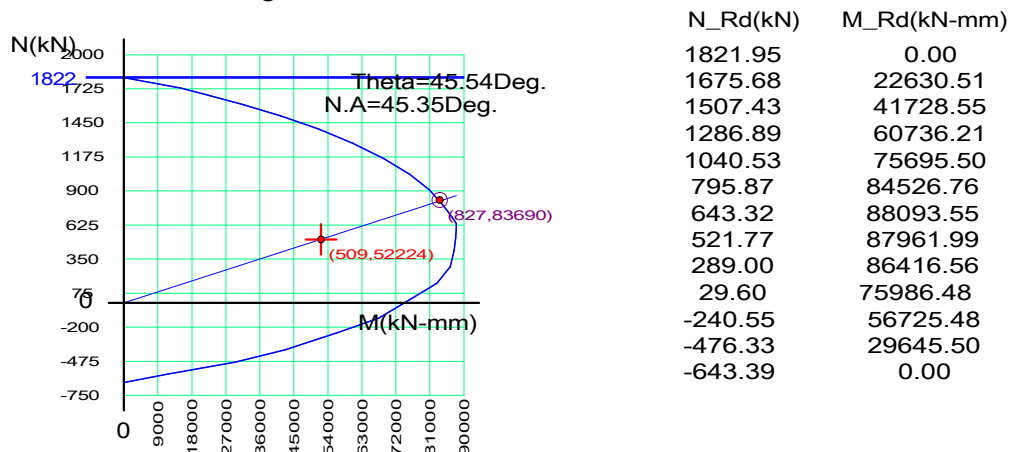
2. Applied Loads

Load Combination : 4 AT (J) Point
 N_{Ed} = 509.254 kN M_{Edy} = 36702.5 kN-mm M_{Edz} = 37152.3 kN-mm
 M_{Ed} = SQRT(M_{Edy}² + M_{Edz}²) = 52224.2 kN-mm

3. Axial Forces and Moments Capacity Check

Concentric Max. Axial Load N_{Rdmax} = 1821.95 kN
 Axial Load Ratio N_{Ed}/N_{Rd} = 509.254 / 826.891 = 0.616 < 1.000 O.K
 Moment Ratio M_{Ed}/M_{Rd} = 52224.2 / 83690.4 = 0.624 < 1.000 O.K
 M_{Edy}/M_{Rdy} = 36702.5 / 58622.7 = 0.626 < 1.000 O.K
 M_{Edz}/M_{Rdz} = 37152.3 / 59728.2 = 0.622 < 1.000 O.K
 Normalized Axial Load Ratio Nu_d / 0.65 = 0.231 / 0.650 = 0.356 < 1.000 O.K

4. M-N Interaction Diagram



5. Shear Force Capacity Check (End)

Applied Shear Force V_{Ed} = 47.2822 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 47.2822 / 82.3687 = 0.574
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 47.2822 / 36.0633 = 1.311
 Shear Ratio V_{Ed}/V_{Rd} = 0.574 < 1.000 O.K
 (Asw-H_{use} = 670.26667 mm²/m, 2-P8 @150)
 Joint Shear Ratio V_{jhd}/V_{js} = 0.00000 / 0.00000 = 0.000 < 1.000 O.K
 (Ash = 268.10667 mm², 2-3 P8)

6. Shear Force Capacity Check (Middle)

Applied Shear Force V_{Ed} = 47.2822 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 47.2822 / 82.3687 = 0.574
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 47.2822 / 36.0633 = 1.311
 Shear Ratio V_{Ed}/V_{Rd} = 0.574 < 1.000 O.K
 (Asw-H_{use} = 670.26667 mm²/m, 2-P8 @150)

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] MESHED WALL DESIGN MAXIMUM RESULT DATA : DOMAIN Water tank-Walls. (Vertical)
 =====

-. Information of Parameters.

Elem No. : 2757
 LCB No. : 4
 Materials : fck = 20000.0000 KPa.
 fyk = 460000.0000 KPa.
 Thickness : t = 0.2000 m.
 Covering : Dw = 0.0250 m.

-. Information of Design.

Alpha_cc = 1.000 (Default or User Defined).
 gamma_c = 1.500 (for Concrete)
 gamma_s = 1.150 (for Reinforcement)
 fcd = Alpha_cc * fck / gamma_c = 13333.3333 KPa.
 fyd = fyk / gamma_s = 400000.0000 KPa.
 Nu = 0.6 * (1 - fck / 250) = 0.5520 (fck in MPa)

-. Design Forces.

Sig_Edx = 1612.1986 KPa.
 Sig_Edy = -610.7701 KPa.
 Tau_Edxy = 359.0605 KPa.

Sig_Ed_max = 1612.1986 KPa. (x-dir)
 Sig_Ed_min = -610.7701 KPa. (y-dir)
 Tau_Edxy = 359.0605 KPa.

(Sig_Ed_min in Tension or Sig_Ed_max * Sig_Ed_min <= Tau_Edxy^2 --> Rebar Required!)

ftd_max = 0.0000 KPa. (x-dir)
 ftd_min = Tau_Edxy^2 / Sig_Ed_max - Sig_Ed_min = 690.7382 KPa. (y-dir)

f'tdx = 0.0000 KPa.
 f'tdy = 690.7382 KPa.
 Sig_cd = Sig_Ed_max * [1 + (Tau_Edxy / Sig_Ed_max)^2] = 1692.1666 KPa.

rhoy_req = max(f'tdy / fyd, 0.002) = 0.0020
 rhox_req = max(f'tdx / fyd, 0.001, 0.25 * rhoy_req) = 0.0010

b = 1.0 m. (by Unit Length).
 Asx_Req = 0.0002 m^2/m. (0.0002 m^2/m.)
 Asy_Req = 0.0004 m^2/m. (0.0004 m^2/m.)
 Asx_use = 0.0004 m^2/m. (0.0004 m^2/m.)
 Asy_use = 0.0006 m^2/m. (0.0006 m^2/m.)
 ftnx = Asx_use / (b * t) * fyd = 785.4000 KPa.
 ftny = Asy_use / (b * t) * fyd = 1131.0000 KPa.

PROJECT TITLE : 50 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Result.

Rein. Bar_x : P10 @200 (Hor.)

Rein. Bar_y : P12 @200 (Ver.)

Rat_x = f'tdx/ftnx = 0.000

Rat_y = f'tdy/ftny = 0.611

Rat_cd = Sig_cd/(Nu*fcd) = 0.230

Rat = Rat_y = 0.611 < 1.0 ---> O.K !

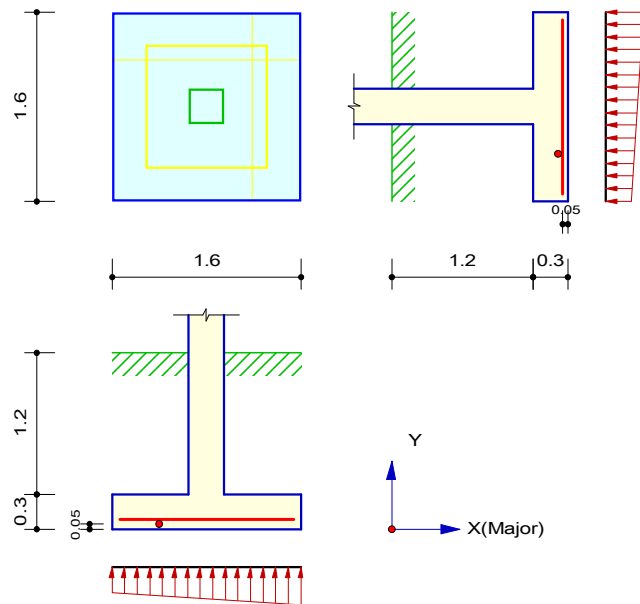
	Company		Project Title	50 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Geometry and Materials

Material : fcu = 20594, fy = 392266 kN/m²
 Dim. : 1.6 * 1.6 * 0.3 m (Dc = 0.05 m)
 Allow. Soil Qe = 350 kN/m²
 Soil Depth H = 1.2 m (Density = 18 kN/m³)

2. Design Condition

Design Code : BS8110-97
 Selected Node No 61
 Design Node No : 61 (Column Size: 0.3*0.3 m)
 Design Load Combination
 Service : 2 [fLCB2 : 1.0D + (1.0LL)]
 Factored : 1 [fLCB1 : 1.35D + 1.5(1.0LL)]
 Applied Loads
 Ns = 421.709, Nu = 593.657 kN
 Msx = 23.3710, Mux = 31.7250 kN-m
 Msy = 23.3421, Muy = 31.6820 kN-m



3. Soil Bearing Pressure Check

Actual Pressure

Qs(max) = 261.957 kN/m² < Qe = 350.000 kN/m² O.K
 Qs(min) = 125.103 kN/m² > 0.00 kN/m² O.K

Design Pressure

Qu(max) = 324.779 kN/m²
 Qu(min) = 139.016 kN/m²

4. Bending Moment Check (Gammamc= 1.50, Gammams= 1.05)

X-X Axis (Y Direction)

	Required Space	Max. Space
Mux = 56.1467 kN-m/m		
As = 0.00064 m ² /m	P12 @ 170	P12 @ 220
As(req) = As*Lx*2/3(Cx+3d) = 0.00065 m ² /m	P13 @ 200	P13 @ 260

Y-Y Axis (X Direction)

	Required Space	Max. Space
Muy = 56.1370 kN-m/m		
As = 0.00067 m ² /m	P12 @ 160	P12 @ 220
As(req) = As*Ly*2/3(Cy+3d) = 0.00071 m ² /m	P13 @ 180	P13 @ 260

5. Shear Check (Gammam= 1.25)

Vertical Shear

Vy_d = 426.802 kN/m² < 2*Vcy = 900.358 kN/m² O.K
 Vy_2d = 164.408 kN/m² < Vcy = 450.179 kN/m² O.K
 Vx_d = 461.087 kN/m² < 2*Vcx = 943.693 kN/m² O.K
 Vx_2d = 199.778 kN/m² < Vcx = 471.846 kN/m² O.K

Punching Shear

V = 584.712 kN/m² < Vc = 691.293 kN/m² O.K
 Vcf = 1956.24 kN/m² < Vmax = 3631.08 kN/m² O.K

Project Name : Elevated Concrete Storage Water Tank (50 cubic meter tank)
Created :02/09/2023
User Name : Sheilla

REQUIRED REINFORCEMENTAND CONCRETE STRENGTH

SECTION	Dimensions	REINFORCEMENT	CONCRETE STRENGTH
Tank roof slab	200mm thick	T10 @ 200 (All bars)	C20/25
Tank floor slab	200 mm thick	T16 @ 200 (All bars)	C20/25
Tank walls	200 mm thick	T12 @ 200 (Vertical) T10 @ 200 (Horizontal)	C20/25
Footing	1.6x1.6x0.3m	T12 @ 150 (All bars)	C20/25