

75 M³ ELEVATED STORAGE WATER TANK DESIGN REPORT

2nd September 2023

**Prepared by:
Sheilla Constance Apio**

ANALYSIS MODEL

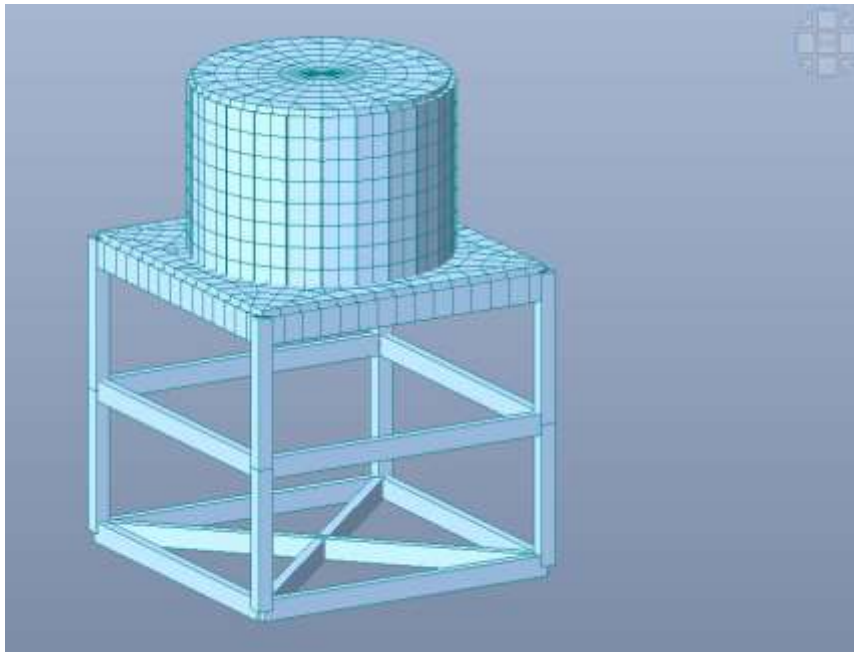


Figure 1 Analysis model

MODAL DISPLACEMENT

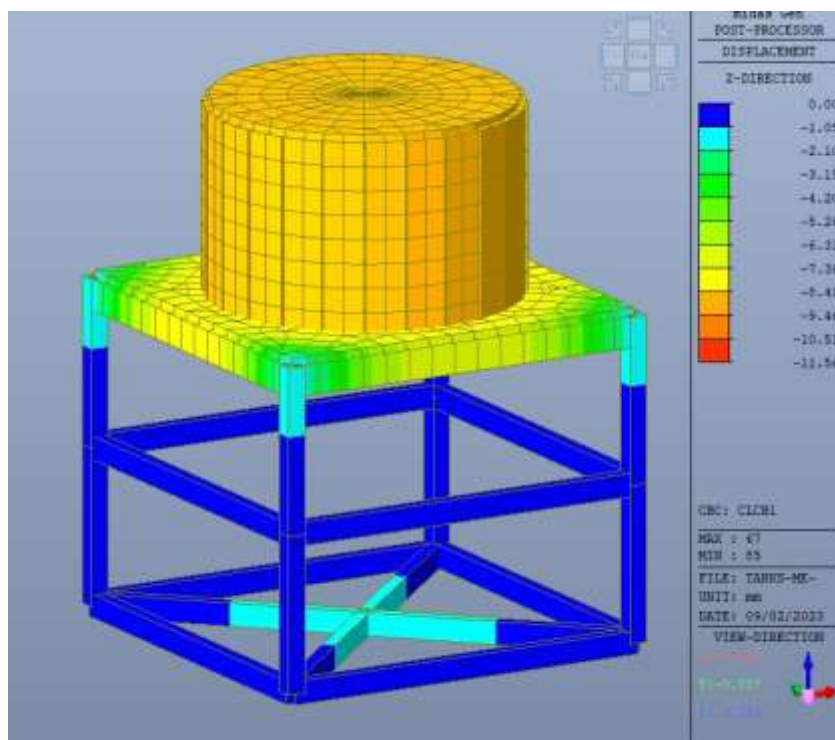


Figure 2 Modal displacement

LOAD CASES AND COMBINATIONS

No.	Name	Combination
1	cLCB1	1.35D + 1.5(1.0LL)
2	cLCB2	1.35D + 1.5(1.0LL) + 1.5(0.6)WL
3	cLCB3	1.35D + 1.5(0.7LL) + 1.5WL
4	cLCB4	1.35D + 1.5(1.0LL) - 1.5(0.6)WL
5	cLCB5	1.35D + 1.5(0.7LL) - 1.5WL
6	cLCB6	1.0D + 1.0(0.3L) + 1.0E
7	cLCB7	1.0D + 1.0(0.3L) - 1.0E
8	cLCB8	SERV :1.0D + (1.0LL)
9	cLCB9	SERV :1.0D + (1.0LL) + (0.6)WL
10	cLCB10	SERV :1.0D + (1.0LL) - (0.6)WL
11	cLCB12	SERV :1.0D + (0.7LL) - 1.0WL
12	cLCB13	SERV :1.0D + (0.5LL)
13	cLCB14	SERV :1.0D + (0.3LL) + (0.2)WL
14	cLCB15	SERV :1.0D + (0.3LL) - (0.2)WL
15	cLCB16	SERV :1.0D + (0.3LL)

D - DEAD LOAD

LL -LIVELOAD

WL - WIND LOAD (based on wind speed of 35 m/s)

E - EARTHQUAKE (based on a PGA of 0.064g)

SERV - Serviceability

LOADS

No.	Section	DEAD LOAD (kN/m2)	LIVE LOAD (kN/m2)
1.	Roof load	-1.2	-1.5

Pressure on the within the tank is based on water density of 10kN/m³

PROJECT TITLE: 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse		M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	1763	BOT	0.0014	0.0020		114.057(1)	155.484	0.734	OK
	1855	TOP	0.0004	0.0020		28.4344(1)	155.484	0.183	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 1763
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.060 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.7969 kN.
 M_Rd = Cc*(d-a/2) = 155.4840 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @100
 As_req = 0.0014 m^2/m. (0.0014 m^2/m.)
 M_Ed = 114.0569 kN-m./m.
 M_Rd = 155.4840 kN-m./m.
 RatM = M_Ed / M_Rd = 0.734 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.335

<< TOP >>

-. Information of Parameters.

Elem No. : 1855
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.060 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.7969 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 155.4840 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @100
 $A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (0.0004 \text{ m}^2/\text{m.})$
 $M_{Ed} = 28.4344 \text{ kN-m./m.}$
 $M_{Rd} = 155.4840 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.183 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.335$

PROJECT TITLE: 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	2030	BOT	0.0014	0.0020	114.027(1)	155.484	0.733	OK
	1777	TOP	0.0004	0.0020	28.4100(1)	155.484	0.183	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 2030
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.060 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.7969 kN.
 M_Rd = Cc*(d-a/2) = 155.4840 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @100
 As_req = 0.0014 m^2/m. (0.0014 m^2/m.)
 M_Ed = 114.0274 kN-m./m.
 M_Rd = 155.4840 kN-m./m.
 RatM = M_Ed / M_Rd = 0.733 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.335

<< TOP >>

-. Information of Parameters.

Elem No. : 1777
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.060 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.7969 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 155.4840 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @100
 $A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (0.0004 \text{ m}^2/\text{m.})$
 $M_{Ed} = 28.4100 \text{ kN-m./m.}$
 $M_{Rd} = 155.4840 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.183 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.335$

PROJECT TITLE: 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	4493	BOT	0.0003	0.0004	10.2866(1)	34.4073	0.299	OK
	4437	TOP	0.0003	0.0004	14.7524(1)	34.4073	0.429	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4493
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1570 kN.
 M_Rd = Cc*(d-a/2) = 34.4073 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0003 m^2/m. (0.0003 m^2/m.)
 M_Ed = 10.2866 kN-m./m.
 M_Rd = 34.4073 kN-m./m.
 RatM = M_Ed / M_Rd = 0.299 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.065

<< TOP >>

-. Information of Parameters.

Elem No. : 4437
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1570 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 34.4073 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0003 \text{ m}^2/\text{m.} \quad (0.0003 \text{ m}^2/\text{m.})$
 $M_{Ed} = 14.7524 \text{ kN-m./m.}$
 $M_{Rd} = 34.4073 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.429 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.065$

PROJECT TITLE: 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	4445	BOT	0.0003	0.0004	10.2950(1)	34.4073	0.299	OK
	4395	TOP	0.0003	0.0004	14.5057(1)	34.4073	0.422	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4445
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1570 kN.
 M_Rd = Cc*(d-a/2) = 34.4073 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0003 m^2/m. (0.0003 m^2/m.)
 M_Ed = 10.2950 kN-m./m.
 M_Rd = 34.4073 kN-m./m.
 RatM = M_Ed / M_Rd = 0.299 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.065

<< TOP >>

-. Information of Parameters.

Elem No. : 4395
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1570 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 34.4073 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0003 \text{ m}^2/\text{m.} \quad (0.0003 \text{ m}^2/\text{m.})$
 $M_{Ed} = 14.5057 \text{ kN-m./m.}$
 $M_{Rd} = 34.4073 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.422 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

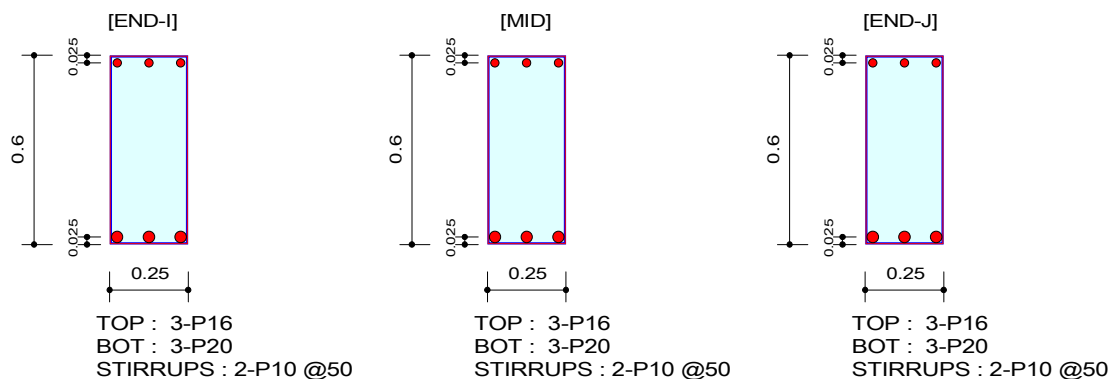
$x/d = 0.065$

	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : fck = 20000, fyk = 460000, fyw = 250000 KPa
 Section Property: 600x250 loading beams (No : 10) Unit System : kN, m
 Beam Span : 10.3945 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	47.49	11.88	47.50
Factored Strength (M_Rd)	133.12	133.12	133.12
Check Ratio (M_Ed/M_Rd)	0.3568	0.0892	0.3568
Neutral Axis (x/d)	0.0579	0.0579	0.0579
(+) Load Combination No.	4	4	4
Moment (M_Ed)	89.66	108.57	89.64
Factored Strength (M_Rd)	207.15	207.15	207.15
Check Ratio (M_Ed/M_Rd)	0.4328	0.5241	0.4327
Neutral Axis (x/d)	0.0977	0.0977	0.0977
Using Rebar Top (As_top)	0.0006	0.0006	0.0006
Using Rebar Bot (As_bot)	0.0009	0.0009	0.0009

4. Shear Capacity

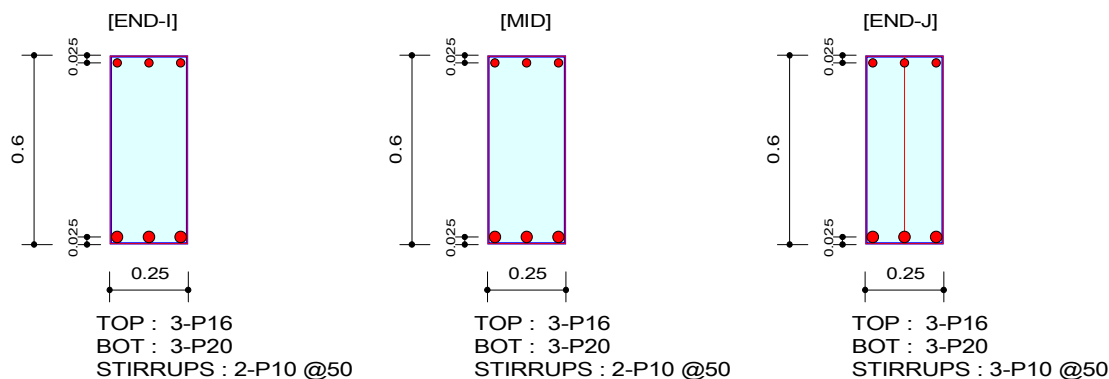
	END-I	MID	END-J
Load Combination No.	4	4	4
Factored Shear Force (V_Ed)	222.15	49.30	222.18
Shear Strength by Conc.(V_Rdc)	64.67	64.67	64.67
Shear Strength by Rebar.(V_Rds)	353.43	353.43	353.43
Shear Strength by Rebar.(V_Rdmax)	476.10	476.10	476.10
Using Shear Reinf. (Asw)	0.0031	0.0031	0.0031
Using Stirrups Spacing	2-P10 @50	2-P10 @50	2-P10 @50
Shear Ratio by Conc	3.4353	0.7623	3.4356
Shear Ratio by (V_Rds ; V_Rdmax)	0.6286	0.1395	0.6286
Check Ratio	0.6286	0.7623	0.6286

	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : fck = 20000, fyk = 460000, fyw = 250000 KPa
 Section Property: 600x250 tie beams (No : 11)
 Unit System : kN, m
 Beam Span : 7.35 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	49.84	12.47	49.87
Factored Strength (M_Rd)	133.12	133.12	133.12
Check Ratio (M_Ed/M_Rd)	0.3744	0.0936	0.3746
Neutral Axis (x/d)	0.0579	0.0579	0.0579
(+) Load Combination No.	4	4	4
Moment (M_Ed)	117.05	144.37	117.07
Factored Strength (M_Rd)	207.15	207.15	207.15
Check Ratio (M_Ed/M_Rd)	0.5651	0.6969	0.5651
Neutral Axis (x/d)	0.0977	0.0977	0.0977
Using Rebar Top (As_top)	0.0006	0.0006	0.0006
Using Rebar Bot (As_bot)	0.0009	0.0009	0.0009

4. Shear Capacity

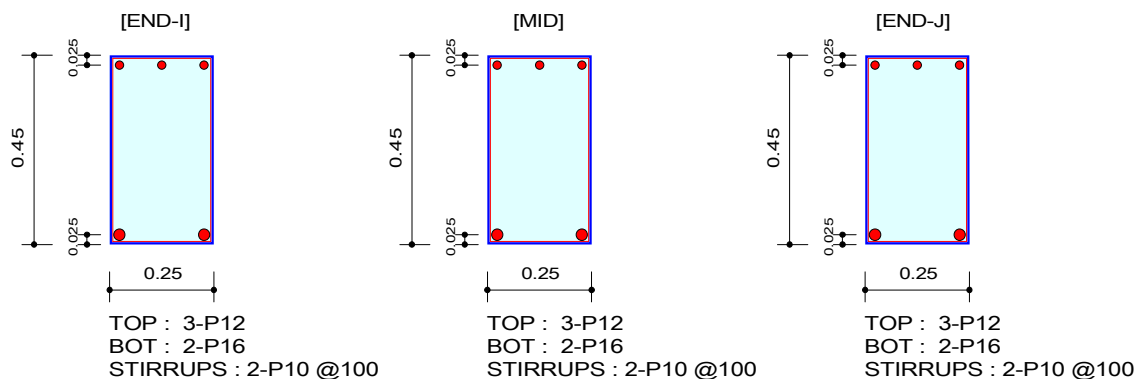
	END-I	MID	END-J
Load Combination No.	4	7	4
Factored Shear Force (V_Ed)	215.29	47.03	215.53
Shear Strength by Conc.(V_Rdc)	64.67	64.67	64.67
Shear Strength by Rebar.(V_Rds)	353.43	353.43	476.10
Shear Strength by Rebar.(V_Rdmax)	476.10	476.10	476.10
Using Shear Reinf. (Asw)	0.0031	0.0031	0.0047
Using Stirrups Spacing	2-P10 @50	2-P10 @50	3-P10 @50
Shear Ratio by Conc	3.3291	0.7273	3.3329
Shear Ratio by (V_Rds ; V_Rdmax)	0.6091	0.1331	0.4527
Check Ratio	0.6091	0.7273	0.4527

	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : $f_{ck} = 20000$, $f_{yk} = 460000$, $f_{yw} = 250000$ KPa
 Section Property: 450x250 tie beams (No : 6) Unit System : kN, m
 Beam Span : 7.35 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	21.69	5.42	21.69
Factored Strength (M_Rd)	55.16	55.16	55.16
Check Ratio (M_Ed/M_Rd)	0.3933	0.0983	0.3932
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	4	4	4
Moment (M_Ed)	10.85	5.42	10.84
Factored Strength (M_Rd)	65.14	65.14	65.14
Check Ratio (M_Ed/M_Rd)	0.1665	0.0833	0.1665
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As_top)	0.0003	0.0003	0.0003
Using Rebar Bot (As_bot)	0.0004	0.0004	0.0004

4. Shear Capacity

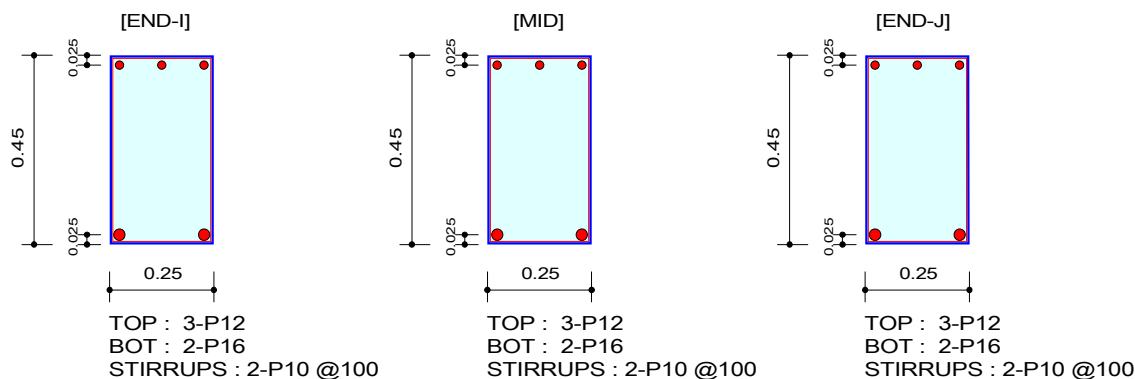
	END-I	MID	END-J
Load Combination No.	7	7	7
Factored Shear Force (V_Ed)	26.70	16.37	26.70
Shear Strength by Conc.(V_Rdc)	42.21	42.21	42.21
Shear Strength by Rebar.(V_Rds)	130.62	130.62	130.62
Shear Strength by Rebar.(V_Rdmax)	351.90	351.90	351.90
Using Shear Reinf. (Asw)	0.0016	0.0016	0.0016
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.6327	0.3878	0.6327
Shear Ratio by (V_Rds ; V_Rdmax)	0.2044	0.1253	0.2044
Check Ratio	0.6327	0.3878	0.6327

	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : $f_{ck} = 20000$, $f_{yk} = 460000$, $f_{yw} = 250000$ KPa
 Section Property: 450x250 ground beams (No : 7) Unit System : kN, m
 Beam Span : 10.3945 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	5	7	5
Moment (M _{Ed})	34.19	0.00	34.19
Factored Strength (M _{Rd})	55.16	55.16	55.16
Check Ratio (M _{Ed} /M _{Rd})	0.6197	0.0000	0.6197
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	5	5	5
Moment (M _{Ed})	4.27	17.09	4.27
Factored Strength (M _{Rd})	65.14	65.14	65.14
Check Ratio (M _{Ed} /M _{Rd})	0.0656	0.2624	0.0656
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As _{top})	0.0003	0.0003	0.0003
Using Rebar Bot (As _{bot})	0.0004	0.0004	0.0004

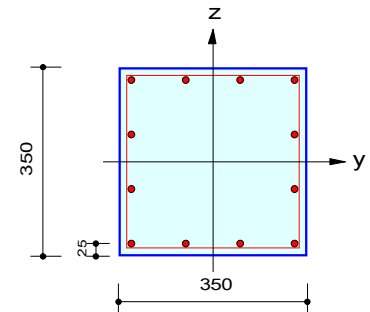
4. Shear Capacity

	END-I	MID	END-J
Load Combination No.	5	5	5
Factored Shear Force (V _{Ed})	19.73	9.87	19.73
Shear Strength by Conc.(V _{Rdc})	42.21	42.21	42.21
Shear Strength by Rebar.(V _{Rds})	130.62	130.62	130.62
Shear Strength by Rebar.(V _{Rdmax})	351.90	351.90	351.90
Using Shear Reinf. (Asw)	0.0016	0.0016	0.0016
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.4675	0.2338	0.4675
Shear Ratio by (V _{Rds} ; V _{Rdmax})	0.1511	0.0755	0.1511
Check Ratio	0.4675	0.2338	0.4675

	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Condition

Design Code : Eurocode2:04 UNIT SYSTEM kN, mm
 Member Number: 108 (PM), 105 (Shear)
 Material Data : fck = 0.02, fyk = 0.46, fyw = 0.25 kN/mm²
 Column Height : 3250 mm
 Section Property: 350x350 columns (No : 3)
 Rebar Pattern : 12 - 4 - P16 Ast = 2412.72 mm² (Rho = 0.020)



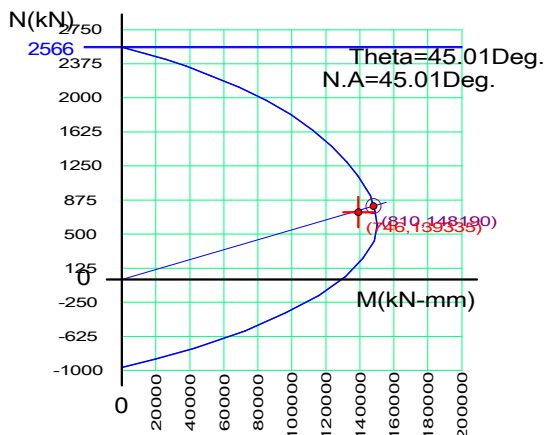
2. Applied Loads

Load Combination : 4 AT (J) Point
 N_{Ed} = 745.757 kN M_{Edy} = 98511.2 kN-mm M_{Edz} = 98538.8 kN-mm
 M_{Ed} = $\sqrt{M_{Edy}^2 + M_{Edz}^2}$ = 139335 kN-mm

3. Axial Forces and Moments Capacity Check

Concentric Max. Axial Load N_{Rdmax} = 2566.25 kN
 Axial Load Ratio N_{Ed}/N_{Rd} = 745.757 / 809.729 = 0.921 < 1.000 O.K
 Moment Ratio M_{Ed}/M_{Rd} = 139335 / 148190 = 0.940 < 1.000 O.K
 M_{Edy}/M_{Rdy} = 98511.2 / 104767 = 0.940 < 1.000 O.K
 M_{Edz}/M_{Rdz} = 98538.8 / 104805 = 0.940 < 1.000 O.K
 Normalized Axial Load Ratio Nu_d / 0.65 = 0.248 / 0.650 = 0.382 < 1.000 O.K

4. M-N Interaction Diagram



N _{Rd} (kN)	M _{Rd} (kN-mm)
2566.25	0.00
2352.01	38047.15
2122.83	68933.50
1822.26	99895.31
1470.54	123630.45
1134.90	139206.09
927.97	146407.80
752.52	148924.55
429.93	148566.86
38.30	131750.32
-365.37	96611.59
-757.51	42840.61
-965.09	0.00

5. Shear Force Capacity Check (End)

Applied Shear Force V_{Ed} = 72.3213 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 72.3213 / 112.926 = 0.640
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 72.3213 / 42.6202 = 1.697
 Shear Ratio V_{Ed}/V_{Rd} = 0.640 < 1.000 O.K
 (Asw-H_{use} = 670.26667 mm²/m, 2-P8 @150)
 Joint Shear Ratio V_{jhd}/V_{js} = 0.00000 / 0.00000 = 0.000 < 1.000 O.K
 (Ash = 268.10667 mm², 2-3 P8)

6. Shear Force Capacity Check (Middle)

Applied Shear Force V_{Ed} = 72.3213 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 72.3213 / 112.926 = 0.640
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 72.3213 / 42.6202 = 1.697
 Shear Ratio V_{Ed}/V_{Rd} = 0.640 < 1.000 O.K
 (Asw-H_{use} = 670.26667 mm²/m, 2-P8 @150)

PROJECT TITLE : 75 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] MESHED WALL DESIGN MAXIMUM RESULT DATA : DOMAIN Water tank-Walls. (Vertical)
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-. Information of Parameters.

Elem No. : 3210
 LCB No. : 4
 Materials : fck = 20000.0000 KPa.
 fyk = 460000.0000 KPa.
 Thickness : t = 0.2500 m.
 Covering : Dw = 0.0250 m.

-. Information of Design.

Alpha_cc = 1.000 (Default or User Defined).
 gamma_c = 1.500 (for Concrete)
 gamma_s = 1.150 (for Reinforcement)
 fcd = Alpha_cc * fck / gamma_c = 13333.3333 KPa.
 fyd = fyk / gamma_s = 400000.0000 KPa.
 Nu = 0.6 * (1 - fck / 250) = 0.5520 (fck in MPa)

-. Design Forces.

Sig_Edx = 1437.7012 KPa.
 Sig_Edy = -864.5665 KPa.
 Tau_Edxy = -313.9477 KPa.

Sig_Ed_max = 1437.7012 KPa. (x-dir)
 Sig_Ed_min = -864.5665 KPa. (y-dir)
 Tau_Edxy = -313.9477 KPa.

(Sig_Ed_min in Tension or Sig_Ed_max * Sig_Ed_min <= Tau_Edxy^2 --> Rebar Required!)

ftd_max = 0.0000 KPa. (x-dir)
 ftd_min = Tau_Edxy^2 / Sig_Ed_max - Sig_Ed_min = 933.1225 KPa. (y-dir)

f'tdx = 0.0000 KPa.
 f'tdy = 933.1225 KPa.
 Sig_cd = Sig_Ed_max * [1 + (Tau_Edxy / Sig_Ed_max)^2] = 1506.2573 KPa.

rhoy_req = max(f'tdy / fyd, 0.002) = 0.0023
 rhox_req = max(f'tdx / fyd, 0.001, 0.25 * rhoy_req) = 0.0010

b = 1.0 m. (by Unit Length).
 Asx_Req = 0.0003 m^2/m. (0.0003 m^2/m.)
 Asy_Req = 0.0006 m^2/m. (0.0006 m^2/m.)
 Asx_use = 0.0004 m^2/m. (0.0004 m^2/m.)
 Asy_use = 0.0008 m^2/m. (0.0008 m^2/m.)
 ftnx = Asx_use / (b * t) * fyd = 628.3200 KPa.
 ftny = Asy_use / (b * t) * fyd = 1206.4000 KPa.

PROJECT TITLE : 75 cum tank

	Company		Client	
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midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Result.
Rein. Bar_x : P10 @200 (Hor.)
Rein. Bar_y : P12 @150 (Ver.)
Rat_x = f'tdx/ftnx = 0.000
Rat_y = f'tdy/ftny = 0.773
Rat_cd = Sig_cd/(Nu*fcd) = 0.205
Rat = Rat_y = 0.773 < 1.0 ---> O.K !

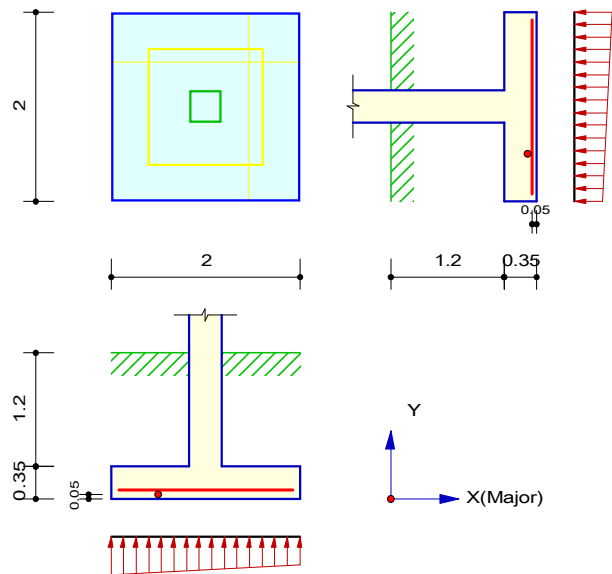
	Company		Project Title	75 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Geometry and Materials

Material : fcu = 20594, fy = 392266 kN/m²
 Dim. : 2 * 2 * 0.35 m (Dc = 0.05 m)
 Allow. Soil Qe = 350 kN/m²
 Soil Depth H = 1.2 m (Density = 18 kN/m³)

2. Design Condition

Design Code : BS8110-97
 Selected Node No 68
 Design Node No : 68 (Column Size: 0.35*0.35 m)
 Design Load Combination
 Service : 2 [fLCB2 : 1.0D + (1.0LL)]
 Factored : 1 [fLCB1 : 1.35D + 1.5(1.0LL)]
 Applied Loads
 Ns = 602.064, Nu = 847.443 kN
 Msx = 35.2843, Mux = 48.1652 kN-m
 Msy = -35.282, Muy = -48.162 kN-m



3. Soil Bearing Pressure Check

Actual Pressure

Qs(max) = 233.441 kN/m² < Qe = 350.000 kN/m² O.K
 Qs(min) = 127.591 kN/m² > 0.00 kN/m² O.K

Design Pressure

Qu(max) = 284.106 kN/m²
 Qu(min) = 139.615 kN/m²

4. Bending Moment Check (Gammamc= 1.50, Gammams= 1.05)

X-X Axis (Y Direction)

	Required Space	Max. Space
Mux = 81.0116 kN-m/m		
As = 0.00076 m ² /m	P12 @ 130	P12 @ 190
As(req) = As*Lx*2/3(Cx+3d) = 0.00081 m ² /m	P13 @ 160	P13 @ 220

Y-Y Axis (X Direction)

	Required Space	Max. Space
Muy = 81.0110 kN-m/m		
As = 0.00080 m ² /m	P12 @ 120	P12 @ 190
As(req) = As*Ly*2/3(Cy+3d) = 0.00088 m ² /m	P13 @ 150	P13 @ 220

5. Shear Check (Gammam= 1.25)

Vertical Shear

Vy_d = 417.379 kN/m² < 2*Vcy = 860.841 kN/m² O.K
 Vy_2d = 182.940 kN/m² < Vcy = 430.420 kN/m² O.K
 Vx_d = 444.300 kN/m² < 2*Vcx = 895.119 kN/m² O.K
 Vx_2d = 210.513 kN/m² < Vcx = 447.559 kN/m² O.K

Punching Shear

V = 599.289 kN/m² < Vc = 658.337 kN/m² O.K
 Vcf = 1995.84 kN/m² < Vmax = 3631.08 kN/m² O.K

Project Name : Elevated Concrete Storage Water Tank (75 cubic meter tank)
Created :02/09/2023
User Name : Sheilla

REQUIRED REINFORCEMENTAND CONCRETE STRENGTH

SECTION	Dimensions	REINFORCEMENT	CONCRETE STRENGTH
Tank roof slab	250mm thick	T10 @ 200 (All bars)	C20/25
Tank floor slab	250 mm thick	T16 @ 100 (All bars)	C20/25
Tank walls	250 mm thick	T12 @ 150 (Vertical) T10 @ 200 (Horizontal)	C20/25
Footing	2.0x2.0x0.35m	T12 @ 125 (All bars)	C20/25