

60 M³ ELEVATED STORAGE WATER TANK DESIGN REPORT

2nd September 2023

Prepared by:
Sheilla Constance Apio

ANALYSIS MODEL

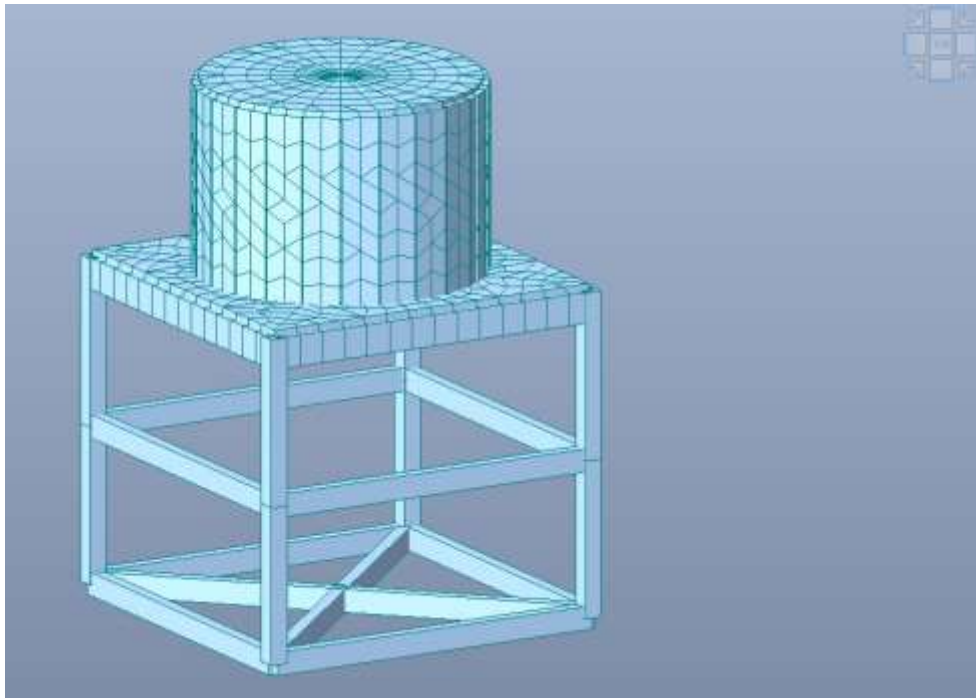


Figure 1 Analysis model

MODAL DISPLACEMENT

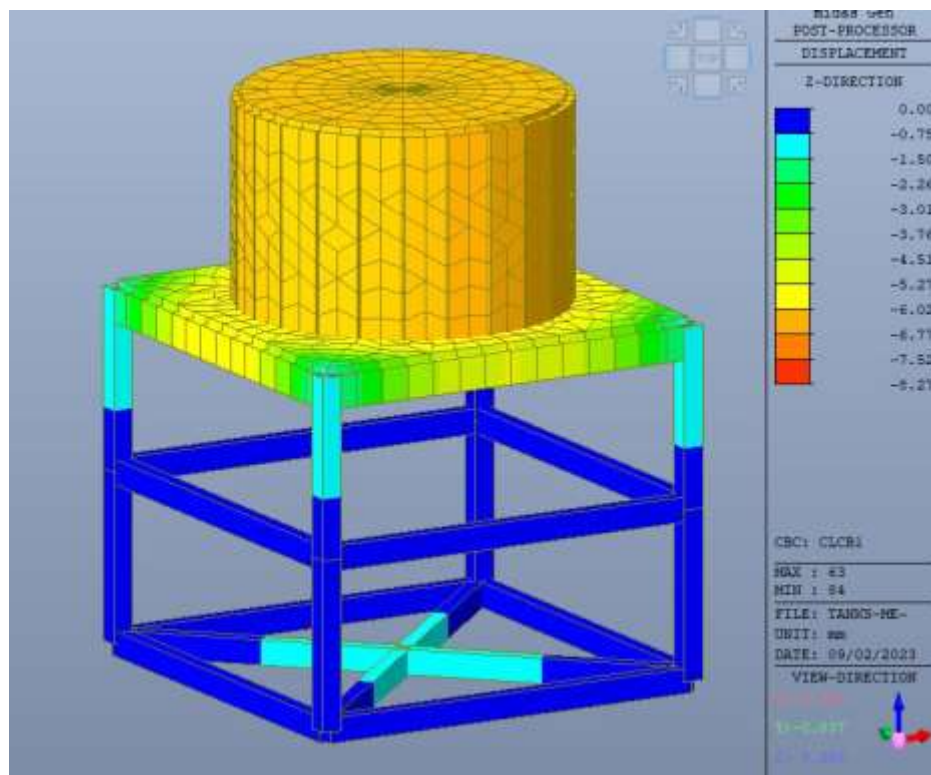


Figure 2 Modal displacement

LOAD CASES AND COMBINATIONS

No.	Name	Combination
1	cLCB1	1.35D + 1.5(1.0LL)
2	cLCB2	1.35D + 1.5(1.0LL) + 1.5(0.6)WL
3	cLCB3	1.35D + 1.5(0.7LL) + 1.5WL
4	cLCB4	1.35D + 1.5(1.0LL) - 1.5(0.6)WL
5	cLCB5	1.35D + 1.5(0.7LL) - 1.5WL
6	cLCB6	1.0D + 1.0(0.3L) + 1.0E
7	cLCB7	1.0D + 1.0(0.3L) - 1.0E
8	cLCB8	SERV :1.0D + (1.0LL)
9	cLCB9	SERV :1.0D + (1.0LL) + (0.6)WL
10	cLCB10	SERV :1.0D + (1.0LL) - (0.6)WL
11	cLCB12	SERV :1.0D + (0.7LL) - 1.0WL
12	cLCB13	SERV :1.0D + (0.5LL)
13	cLCB14	SERV :1.0D + (0.3LL) + (0.2)WL
14	cLCB15	SERV :1.0D + (0.3LL) - (0.2)WL
15	cLCB16	SERV :1.0D + (0.3LL)

D - DEAD LOAD

LL -LIVELOAD

WL - WIND LOAD (based on wind speed of 35 m/s)

E - EARTHQUAKE (based on a PGA of 0.064g)

SERV - Serviceability

LOADS

No.	Section	DEAD LOAD (kN/m ²)	LIVE LOAD (kN/m ²)
1.	Roof load	-1.2	-1.5

Pressure on the within the tank is based on water density of 10kN/m³

PROJECT TITLE: 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	1479	BOT	0.0012	0.0020	99.7026(1)	155.484	0.641	OK
	1513	TOP	0.0003	0.0020	24.3879(1)	155.484	0.157	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 1479
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.060 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.7969 kN.
 M_Rd = Cc*(d-a/2) = 155.4840 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @100
 As_req = 0.0012 m^2/m. (0.0012 m^2/m.)
 M_Ed = 99.7026 kN-m./m.
 M_Rd = 155.4840 kN-m./m.
 RatM = M_Ed / M_Rd = 0.641 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.335

<< TOP >>

-. Information of Parameters.

Elem No. : 1513
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.060 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.7969 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 155.4840 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @100
 $A_{s_req} = 0.0003 \text{ m}^2/\text{m.} \quad (0.0003 \text{ m}^2/\text{m.})$
 $M_{Ed} = 24.3879 \text{ kN-m./m.}$
 $M_{Rd} = 155.4840 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.157 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.335$

PROJECT TITLE: 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Bottom slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2500	1534	BOT	0.0012	0.0020	99.7879(1)	155.484	0.642	OK
	1475	TOP	0.0003	0.0020	24.3726(1)	155.484	0.157	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 1534
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.2250 m.
 lambda = 0.800
 a = lambda * x = 0.060 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.7969 kN.
 M_Rd = Cc*(d-a/2) = 155.4840 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P16 @100
 As_req = 0.0012 m^2/m. (0.0012 m^2/m.)
 M_Ed = 99.7879 kN-m./m.
 M_Rd = 155.4840 kN-m./m.
 RatM = M_Ed / M_Rd = 0.642 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.335

<< TOP >>

-. Information of Parameters.

Elem No. : 1475
 Thickness : 0.2500 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.2250 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.060 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.7969 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 155.4840 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P16 @100
 $A_{s_req} = 0.0003 \text{ m}^2/\text{m.} \quad (0.0003 \text{ m}^2/\text{m.})$
 $M_{Ed} = 24.3726 \text{ kN-m./m.}$
 $M_{Rd} = 155.4840 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.157 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.335$

PROJECT TITLE: 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 1.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	4353	BOT	0.0002	0.0004	6.79819(1)	26.5133	0.256	OK
	4318	TOP	0.0002	0.0004	10.8976(1)	26.5133	0.411	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4353
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1568 kN.
 M_Rd = Cc*(d-a/2) = 26.5133 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0002 m^2/m. (0.0002 m^2/m.)
 M_Ed = 6.7982 kN-m./m.
 M_Rd = 26.5133 kN-m./m.
 RatM = M_Ed / M_Rd = 0.256 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.084

<< TOP >>

-. Information of Parameters.

Elem No. : 4318
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1568 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 26.5133 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0002 \text{ m}^2/\text{m.} \quad (0.0002 \text{ m}^2/\text{m.})$
 $M_{Ed} = 10.8976 \text{ kN-m./m.}$
 $M_{Rd} = 26.5133 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.411 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

$x/d = 0.084$

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] SLAB CHECKING MAXIMUM RESULT DATA : DOMAIN Water tank-Top slab, Dir 2.
 =====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
0.2000	4305	BOT	0.0002	0.0004	6.80093(1)	26.5133	0.257	OK
	4249	TOP	0.0002	0.0004	10.7783(1)	26.5133	0.407	OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 4305
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).
 d = 0.1750 m.
 lambda = 0.800
 a = lambda * x = 0.012 m.
 eta = 1.000
 Cc = eta*fcd*b*a = 0.1568 kN.
 M_Rd = Cc*(d-a/2) = 26.5133 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P10 @200
 As_req = 0.0002 m^2/m. (0.0002 m^2/m.)
 M_Ed = 6.8009 kN-m./m.
 M_Rd = 26.5133 kN-m./m.
 RatM = M_Ed / M_Rd = 0.257 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.084

<< TOP >>

-. Information of Parameters.

Elem No. : 4249
 Thickness : 0.2000 m.
 Materials : fck = 20000.0000 KPa.
 fcd = 13333.3333 KPa.
 fyk = 460000.0000 KPa.
 Covering : dB = 0.0250 m.
 dT = 0.0250 m.
 LCB No. : 1

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Slab Flexural Checking [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Design.

$b = 0.0010 \text{ m. (by Code Unit Length).}$
 $d = 0.1750 \text{ m.}$
 $\lambda = 0.800$
 $a = \lambda * x = 0.012 \text{ m.}$
 $\eta = 1.000$
 $C_c = \eta * f_{cd} * b * a = 0.1568 \text{ kN.}$
 $M_{Rd} = C_c * (d - a/2) = 26.5133 \text{ kN-m./m.}$

-. Information of Moments and Result.

Rein. Bar : P10 @200
 $A_{s_req} = 0.0002 \text{ m}^2/\text{m.} \quad (0.0002 \text{ m}^2/\text{m.})$
 $M_{Ed} = 10.7783 \text{ kN-m./m.}$
 $M_{Rd} = 26.5133 \text{ kN-m./m.}$
 $RatM = M_{Ed} / M_{Rd} = 0.407 < 1.0 \text{ ---> O.K !}$

-. Check ratio of neutral axis depth to effective depth.

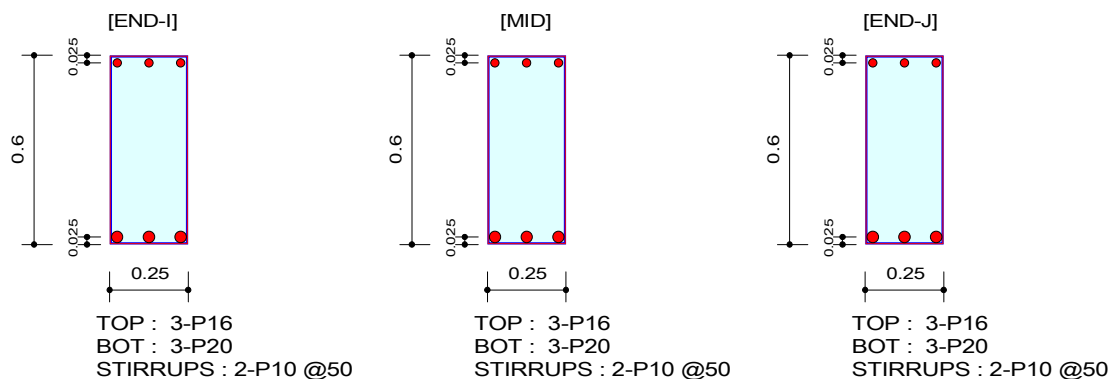
$x/d = 0.084$

	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : fck = 20000, fyk = 460000, fyw = 250000 KPa
 Section Property: 600x250 loading beams (No : 10) Unit System : kN, m
 Beam Span : 9.54594 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M _{Ed})	39.86	9.97	39.89
Factored Strength (M _{Rd})	133.12	133.12	133.12
Check Ratio (M _{Ed} /M _{Rd})	0.2994	0.0749	0.2997
Neutral Axis (x/d)	0.0579	0.0579	0.0579
(+) Load Combination No.	4	4	4
Moment (M _{Ed})	75.39	89.81	75.65
Factored Strength (M _{Rd})	207.15	207.15	207.15
Check Ratio (M _{Ed} /M _{Rd})	0.3639	0.4336	0.3652
Neutral Axis (x/d)	0.0977	0.0977	0.0977
Using Rebar Top (As _{top})	0.0006	0.0006	0.0006
Using Rebar Bot (As _{bot})	0.0009	0.0009	0.0009

4. Shear Capacity

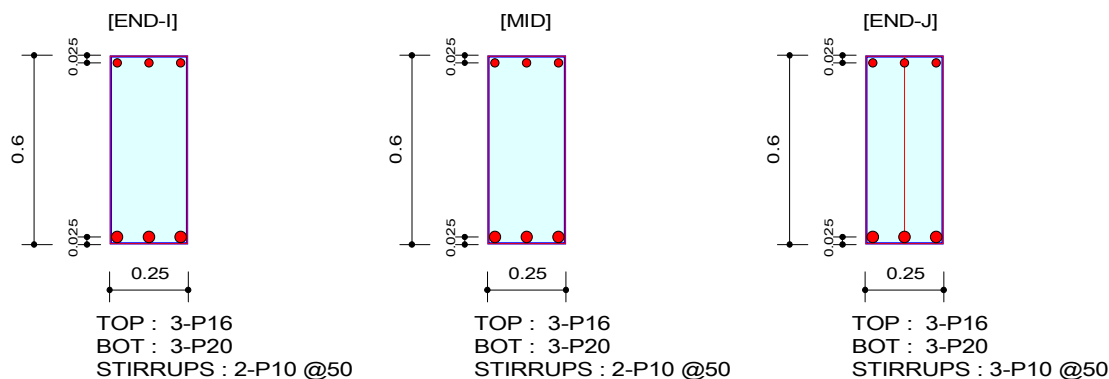
	END-I	MID	END-J
Load Combination No.	4	4	4
Factored Shear Force (V _{Ed})	187.88	49.30	188.28
Shear Strength by Conc.(V _{Rdc})	64.67	64.67	64.67
Shear Strength by Rebar.(V _{Rds})	353.43	353.43	353.43
Shear Strength by Rebar.(V _{Rdmax})	476.10	476.10	476.10
Using Shear Reinf. (Asw)	0.0031	0.0031	0.0031
Using Stirrups Spacing	2-P10 @50	2-P10 @50	2-P10 @50
Shear Ratio by Conc	2.9054	0.7623	2.9115
Shear Ratio by (V _{Rds} ; V _{Rdmax})	0.5316	0.1395	0.5327
Check Ratio	0.5316	0.7623	0.5327

	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : fck = 20000, fyk = 460000, fyw = 250000 KPa
 Section Property: 600x250 tie beams (No : 11)
 Unit System : kN, m
 Beam Span : 7.35 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	49.84	12.47	49.87
Factored Strength (M_Rd)	133.12	133.12	133.12
Check Ratio (M_Ed/M_Rd)	0.3744	0.0936	0.3746
Neutral Axis (x/d)	0.0579	0.0579	0.0579
(+) Load Combination No.	4	4	4
Moment (M_Ed)	117.05	144.37	117.07
Factored Strength (M_Rd)	207.15	207.15	207.15
Check Ratio (M_Ed/M_Rd)	0.5651	0.6969	0.5651
Neutral Axis (x/d)	0.0977	0.0977	0.0977
Using Rebar Top (As_top)	0.0006	0.0006	0.0006
Using Rebar Bot (As_bot)	0.0009	0.0009	0.0009

4. Shear Capacity

	END-I	MID	END-J
Load Combination No.	4	7	4
Factored Shear Force (V_Ed)	215.29	54.98	215.53
Shear Strength by Conc.(V_Rdc)	64.67	64.67	64.67
Shear Strength by Rebar.(V_Rds)	353.43	353.43	476.10
Shear Strength by Rebar.(V_Rdmax)	476.10	476.10	476.10
Using Shear Reinf. (Asw)	0.0031	0.0031	0.0047
Using Stirrups Spacing	2-P10 @50	2-P10 @50	3-P10 @50
Shear Ratio by Conc	3.3291	0.8502	3.3329
Shear Ratio by (V_Rds ; V_Rdmax)	0.6091	0.1556	0.4527
Check Ratio	0.6091	0.8502	0.4527

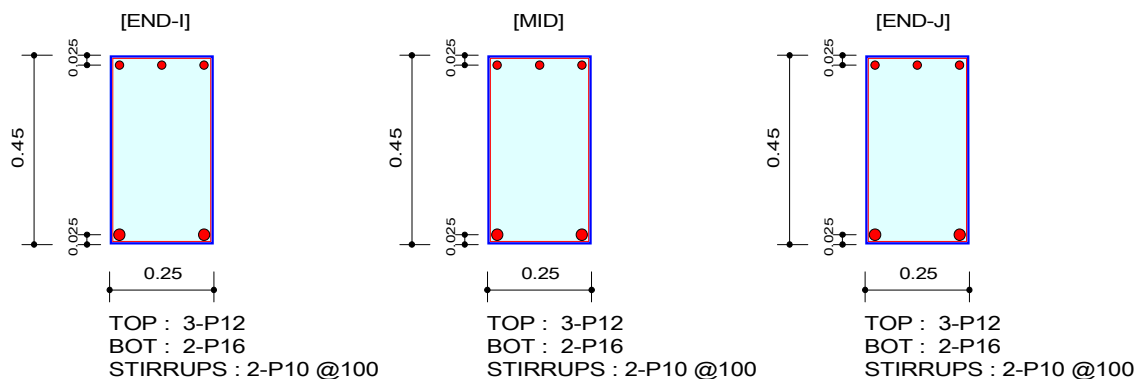
RC Beam Strength Checking Result

	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code	: Eurocode2:04	Unit System	: kN, m
Material Data	: fck = 20000, fyk = 460000, fyw = 250000 KPa		
Section Property:	450x250 tie beams (No : 6)	Beam Span	: 6.75 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	4	4	4
Moment (M_Ed)	17.85	4.46	17.84
Factored Strength (M_Rd)	55.16	55.16	55.16
Check Ratio (M_Ed/M_Rd)	0.3235	0.0809	0.3235
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	4	5	4
Moment (M_Ed)	8.92	4.63	8.92
Factored Strength (M_Rd)	65.14	65.14	65.14
Check Ratio (M_Ed/M_Rd)	0.1370	0.0711	0.1370
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As_top)	0.0003	0.0003	0.0003
Using Rebar Bot (As_bot)	0.0004	0.0004	0.0004

4. Shear Capacity

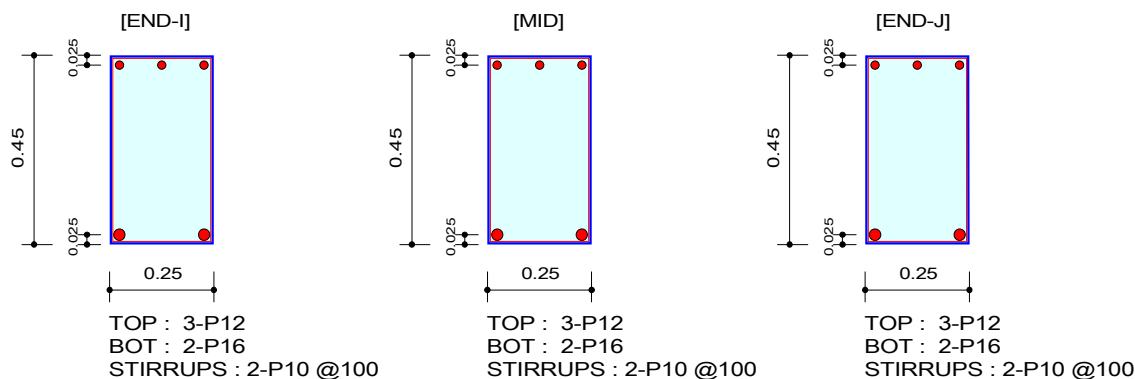
	END-I	MID	END-J
Load Combination No.	7	7	7
Factored Shear Force (V_Ed)	27.31	17.82	27.31
Shear Strength by Conc.(V_Rdc)	42.21	42.21	42.21
Shear Strength by Rebar.(V_Rds)	130.62	130.62	130.62
Shear Strength by Rebar.(V_Rdmax)	351.90	351.90	351.90
Using Shear Reinf. (Asw)	0.0016	0.0016	0.0016
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.6472	0.4223	0.6472
Shear Ratio by (V_Rds ; V_Rdmax)	0.2091	0.1365	0.2091
Check Ratio	0.6472	0.4223	0.6472

	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Information

Design Code : Eurocode2:04
 Material Data : $f_{ck} = 20000$, $f_{yk} = 460000$, $f_{yw} = 250000$ KPa
 Section Property: 450x250 ground beams (No : 7) Unit System : kN, m
 Beam Span : 9.54594 m

2. Section Diagram



3. Bending Moment Capacity

	END-I	MID	END-J
(-) Load Combination No.	5	7	5
Moment (M _{Ed})	28.83	0.00	28.83
Factored Strength (M _{Rd})	55.16	55.16	55.16
Check Ratio (M _{Ed} /M _{Rd})	0.5227	0.0000	0.5227
Neutral Axis (x/d)	0.0725	0.0725	0.0725
(+) Load Combination No.	5	5	5
Moment (M _{Ed})	3.60	14.42	3.60
Factored Strength (M _{Rd})	65.14	65.14	65.14
Check Ratio (M _{Ed} /M _{Rd})	0.0553	0.2213	0.0553
Neutral Axis (x/d)	0.0835	0.0835	0.0835
Using Rebar Top (As _{top})	0.0003	0.0003	0.0003
Using Rebar Bot (As _{bot})	0.0004	0.0004	0.0004

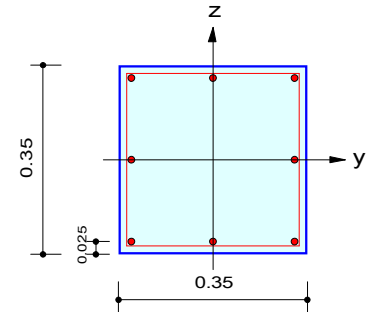
4. Shear Capacity

	END-I	MID	END-J
Load Combination No.	5	5	5
Factored Shear Force (V _{Ed})	18.12	9.06	18.12
Shear Strength by Conc.(V _{Rdc})	42.21	42.21	42.21
Shear Strength by Rebar.(V _{Rds})	130.62	130.62	130.62
Shear Strength by Rebar.(V _{Rdmax})	351.90	351.90	351.90
Using Shear Reinf. (Asw)	0.0016	0.0016	0.0016
Using Stirrups Spacing	2-P10 @100	2-P10 @100	2-P10 @100
Shear Ratio by Conc	0.4294	0.2147	0.4294
Shear Ratio by (V _{Rds} ; V _{Rdmax})	0.1387	0.0694	0.1387
Check Ratio	0.4294	0.2147	0.4294

	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Design Condition

Design Code : Eurocode2:04 UNIT SYSTEM kN, m
 Member Number: 98 (PM), 97 (Shear)
 Material Data : fck = 20000, fyk = 460000, fyw = 250000 KPa
 Column Height : 3.25 m
 Section Property: 350x350 columns (No : 3)
 Rebar Pattern : 8 - 3 - P16 Ast = 0.00160848 m² (Rho_{st} = 0.013)



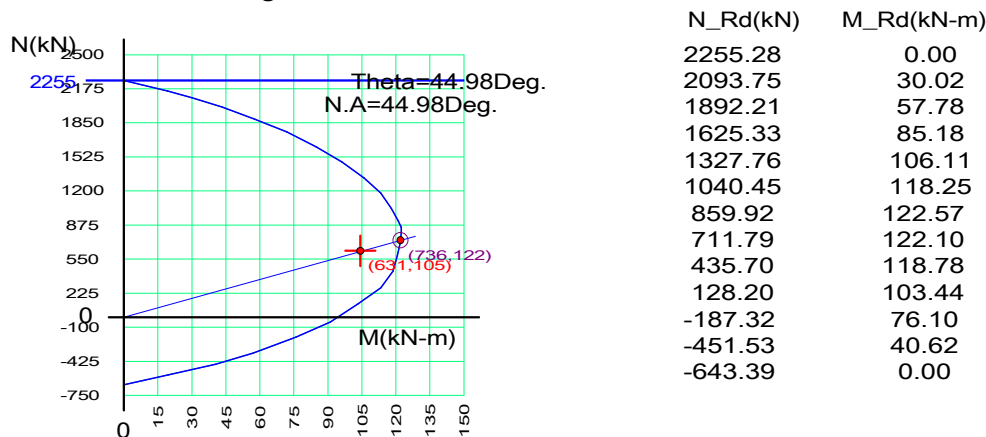
2. Applied Loads

Load Combination : 4 AT (J) Point
 N_{Ed} = 630.914 kN M_{Edy} = 74.0150 kN-m M_{Edz} = 73.9733 kN-m
 M_{Ed} = SQRT(M_{Edy}² + M_{Edz}²) = 104.644 kN-m

3. Axial Forces and Moments Capacity Check

Concentric Max. Axial Load N_{Rdmax} = 2255.28 kN
 Axial Load Ratio N_{Ed}/N_{Rd} = 630.914 / 735.784 = 0.857 < 1.000 O.K
 Moment Ratio M_{Ed}/M_{Rd} = 104.644 / 122.250 = 0.856 < 1.000 O.K
 M_{Edy}/M_{Rdy} = 74.0150 / 86.4764 = 0.856 < 1.000 O.K
 M_{Edz}/M_{Rdz} = 73.9733 / 86.4110 = 0.856 < 1.000 O.K
 Normalized Axial Load Ratio Nu_d / 0.65 = 0.214 / 0.650 = 0.329 < 1.000 O.K

4. M-N Interaction Diagram



5. Shear Force Capacity Check (End)

Applied Shear Force V_{Ed} = 58.8340 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 58.8340 / 104.402 = 0.564
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 58.8340 / 42.6202 = 1.380
 Shear Ratio V_{Ed}/V_{Rd} = 0.564 < 1.000 O.K
 (A_{sw}-H_{use} = 0.00067 m²/m, 2-P8 @150)
 Joint Shear Ratio V_{jhd}/V_{js} = 0.00000 / 0.00000 = 0.000 < 1.000 O.K
 (A_{sh} = 0.00027 m², 2-3 P8)

6. Shear Force Capacity Check (Middle)

Applied Shear Force V_{Ed} = 58.8340 kN (Load Combination : 7)
 Shear Ratio by Conc V_{Ed}/V_{Rdc} = 58.8340 / 104.402 = 0.564
 Shear Ratio by (V_{Rds} ; V_{Rdmax}) V_{Ed}/V_{Rds} = 58.8340 / 42.6202 = 1.380
 Shear Ratio V_{Ed}/V_{Rd} = 0.564 < 1.000 O.K
 (A_{sw}-H_{use} = 0.00067 m²/m, 2-P8 @150)

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019

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 [[[*]]] MESHED WALL DESIGN MAXIMUM RESULT DATA : DOMAIN Water tank-Walls. (Vertical)
 =====

-. Information of Parameters.

Elem No. : 3159
 LCB No. : 4
 Materials : fck = 20000.0000 KPa.
 fyk = 460000.0000 KPa.
 Thickness : t = 0.2500 m.
 Covering : Dw = 0.0250 m.

-. Information of Design.

Alpha_cc = 1.000 (Default or User Defined).
 gamma_c = 1.500 (for Concrete)
 gamma_s = 1.150 (for Reinforcement)
 fcd = Alpha_cc * fck / gamma_c = 13333.3333 KPa.
 fyd = fyk / gamma_s = 400000.0000 KPa.
 Nu = 0.6 * (1 - fck / 250) = 0.5520 (fck in MPa)

-. Design Forces.

Sig_Edx = 473.1683 KPa.
 Sig_Edy = -425.6683 KPa.
 Tau_Edxy = 476.4071 KPa.

Sig_Ed_max = 473.1683 KPa. (x-dir)
 Sig_Ed_min = -425.6683 KPa. (y-dir)
 Tau_Edxy = 476.4071 KPa.

(Sig_Ed_min in Tension or Sig_Ed_max * Sig_Ed_min <= Tau_Edxy^2 --> Rebar Required!)

ftd_max = |Tau_Edxy| - Sig_Ed_max = 3.2389 KPa. (x-dir)
 ftd_min = |Tau_Edxy| - Sig_Ed_min = 902.0754 KPa. (y-dir)

f'tdx = 3.2389 KPa.
 f'tdy = 902.0754 KPa.
 Sig_cd = 2 * |Tau_Edxy| = 952.8142 KPa.

rhoy_req = max(f'tdy/fyd, 0.002) = 0.0023
 rhox_req = max(f'tdx/fyd, 0.001, 0.25 * rhoy_req) = 0.0010

b = 1.0 m. (by Unit Length).
 Asx_Req = 0.0003 m^2/m. (0.0003 m^2/m.)
 Asy_Req = 0.0006 m^2/m. (0.0006 m^2/m.)
 Asx_use = 0.0004 m^2/m. (0.0004 m^2/m.)
 Asy_use = 0.0006 m^2/m. (0.0006 m^2/m.)
 ftnx = Asx_use / (b * t) * fyd = 628.3200 KPa.
 ftny = Asy_use / (b * t) * fyd = 904.8000 KPa.

PROJECT TITLE : 60 cum tank

	Company		Client	
	Author	Sheilla CA	File Name	tanks-meshed-loaded-analysis.rcs

midas Gen - RC-Mesh Flexural Wall Design [Eurocode2:04 & Eurocode8:04] Gen 2019
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-. Information of Result.
Rein. Bar_x : P10 @200 (Hor.)
Rein. Bar_y : P12 @200 (Ver.)
Rat_x = f'tdx/ftnx = 0.005
Rat_y = f'tdy/ftny = 0.997
Rat_cd = Sig_cd/(Nu*fcd) = 0.129
Rat = Rat_y = 0.997 < 1.0 ---> O.K !

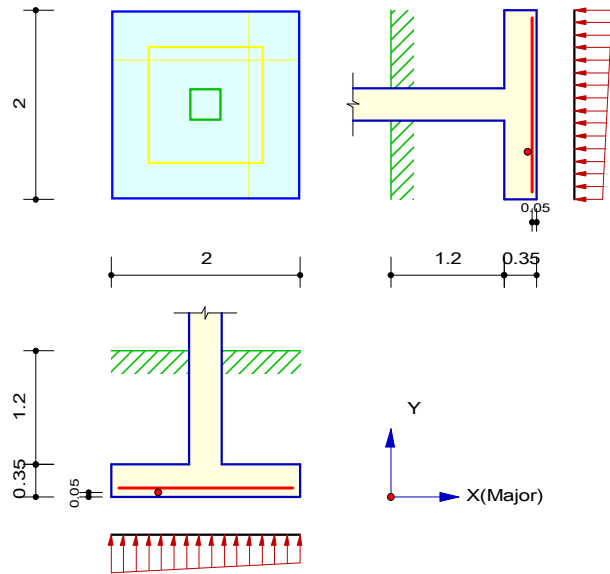
	Company		Project Title	60 cum tank
	Author	Sheilla C A	File Name	D:\...s-meshed-loaded-analysis.mgb

1. Geometry and Materials

Material : fcu = 20594, fy = 392266 kN/m²
 Dim. : 2 * 2 * 0.35 m (Dc = 0.05 m)
 Allow. Soil Qe = 350 kN/m²
 Soil Depth H = 1.2 m (Density = 18 kN/m³)

2. Design Condition

Design Code : BS8110-97
 Selected Node No 64
 Design Node No : 64 (Column Size: 0.35*0.35 m)
 Design Load Combination
 Service : 2 [fLCB2 : 1.0D + (1.0LL)]
 Factored : 1 [fLCB1 : 1.35D + 1.5(1.0LL)]
 Applied Loads
 Ns = 516.562, Nu = 726.339 kN
 Msx = 28.9889, Mux = 39.5225 kN-m
 Msy = -28.991, Muy = -39.526 kN-m



3. Soil Bearing Pressure Check

Actual Pressure

Qs(max) = 202.625 kN/m² < Qe = 350.000 kN/m² O.K
 Qs(min) = 115.656 kN/m² > 0.00 kN/m² O.K

Design Pressure

Qu(max) = 240.871 kN/m²
 Qu(min) = 122.299 kN/m²

4. Bending Moment Check (Gammamc= 1.50, Gammams= 1.05)

X-X Axis (Y Direction)

	Required Space	Max. Space
Mux = 69.1090 kN-m/m		
As = 0.00065 m ² /m	P12 @ 160	P12 @ 190
As(req) = As*Lx*2/3(Cx+3d) = 0.00069 m ² /m	P13 @ 190	P13 @ 220

Y-Y Axis (X Direction)

	Required Space	Max. Space
Muy = 69.1096 kN-m/m		
As = 0.00068 m ² /m	P12 @ 150	P12 @ 190
As(req) = As*Ly*2/3(Cy+3d) = 0.00074 m ² /m	P13 @ 170	P13 @ 220

5. Shear Check (Gammam= 1.25)

Vertical Shear

Vy_d = 356.030 kN/m² < 2*Vcy = 816.090 kN/m² O.K
 Vy_2d = 155.919 kN/m² < Vcy = 408.045 kN/m² O.K
 Vx_d = 379.013 kN/m² < 2*Vcx = 847.209 kN/m² O.K
 Vx_2d = 179.434 kN/m² < Vcx = 423.605 kN/m² O.K

Punching Shear

V = 513.648 kN/m² < Vc = 623.596 kN/m² O.K
 Vcf = 1710.63 kN/m² < Vmax = 3631.08 kN/m² O.K

Project Name : Elevated Concrete Storage Water Tank (60 cubic meter tank)
Created :02/09/2023
User Name : Sheilla

REQUIRED REINFORCEMENTAND CONCRETE STRENGTH

SECTION	Dimensions	REINFORCEMENT	CONCRETE STRENGTH
Tank roof slab	200 mm thick	T10 @ 200 (All bars)	C20/25
Tank floor slab	200 mm thick	T16 @ 100 (All bars)	C20/25
Tank walls	250 mm thick	T12 @ 200 (Vertical) T10 @ 200 (Horizontal)	C20/25
Footing	2.0x2.0x0.35m	T12 @ 150 (All bars)	C20/25